



# Challenges and Innovations in Geotechnics

Editor: Askar Zhussupbekov



## Table of contents

<i>Preface</i>	IX
<i>Conference organizers</i>	XI
<i>Supporting organizations</i>	XIII
 <i>Special Lecturers</i>	
Future challenges for geotechnical engineers (a more contractor orientated perspective) <i>W. Sondermann &amp; C. Kummerer</i>	3
Reconnaissance in and findings from the 2016 Kumamoto earthquake in Japan <i>I. Towhata</i>	9
 <i>Keynote Lecturers</i>	
Application of geosynthetics in civil engineers projects <i>E.C. Shin, M.K. Jung &amp; J.K. Kang</i>	15
Problems of Syrian historical monuments, destroyed by military action: Diagnostics, strengthening and reconstruction <i>T. Awwad</i>	21
Geotechnical and construction considerations of pile foundations in problematical soils <i>A.Zh. Zhussupbekov &amp; A.R. Omarov</i>	27
On interim results of sand production research in weak sandstone formations in Kazakhstan <i>J.R. Kim &amp; M.H. Nguyen</i>	33
 <i>Laboratory and field testing (TS1)</i>	
Centrifuge applications in micropile foundations <i>A. Alnuaim, M.H. El Naggar &amp; H. El Naggar</i>	41
Analyzing of soil ground to frost heaving of structures <i>Z. Shakhmov, A. Tleubayeva, E. Smagulova, L. Utepbergenova, Y. Togabayev &amp; D. Bazarbayev</i>	47
Advantages of the piles testing methods according to the USA standards <i>A.S. Tulebekova, N.T. Alibekova, I.T. Zhumadilov &amp; G.R. Alipbayeva</i>	51
Measured temperature and moisture distribution in the subgrade of the “Almaty-Bishkek” highway <i>A. Nugmanova &amp; B. Teltayev</i>	57
Research of the mechanical properties of soil basis by an equivalent material <i>B. Kaldanova, G. Zhukanova, A.Zh. Zhussupbekov, Y. Muzdybayev, T. Muzdybayeva &amp; B. Dosmukhambetova</i>	61
Evaluation of wind power unit reliability according to the results of field studies on the example of Ereymentau wind power station <i>D.K. Orazova, A.Zh. Zhussupbekov, R.E. Lukpanov &amp; S.B. Yenkebayev</i>	65
Influence of blow energy of the hammer on the bearing capacity of piles during dynamic testing <i>R.E. Lukpanov, D.V. Tsigulyov, S.B. Yenkebayev &amp; D.T. Askarov</i>	71
 <i>Foundation and underground structure (TS2)</i>	
A case study on zoned excavation of a large foundation pit in proximity to existing tunnels <i>Q. Li, Z.H. Xu, J. Li &amp; X.J. Wang</i>	77

## Evaluation of wind power unit reliability according to the results of field studies on the example of Ereymentau wind power station

D.K. Orazova, A.Zh. Zhussupbekov, R.E. Lukpanov & S.B. Yenkebayev

Department of Civil Engineering, Eurasian National University, Astana, Kazakhstan

**ABSTRACT:** This paper includes the analysis of vibrational effect from the tower block to the foundation of the wind generating unit of Ereymentau WPS. The paper shows results of vibromonitoring of wind power tower (WPT). The analysis of vibrating influence from the tower fluctuating load on WPU foundation had been made by the results of in-situ monitoring. The results of research are presented in diagram dependence of vibrating characteristics (frequency, amplitude, acceleration) from wind pressure intensity. There was given an extrapolation of potential efforts arising in the foundation at maximum wind load in that region. Plaxis 2D software program shows the calculation of vibration impacts corresponding to the foundation of wind power unit (WPU) from the tower.

### 1 INTRODUCTION

Wind power is the most dynamically developing type of renewable energy sources. Having studied the energy potential of wind in Kazakhstan, the Government of the Republic of Kazakhstan together with the UN development Program "Kazakhstan is the initiative of the development of wind power market" has resolved that the Ereymentau district of Akmola region is the most perspective area for the construction of wind power stations [1].

The first steps in the Program realization were taken in Ereymentau district of Akmola region.

Within the context of an upcoming exhibition "EXPO-2017" in Astana it is planned to provide power supply of the exhibition facilities by using the energy of Ereymentau WPS.

### 2 CLIMATIC AND HYDROGEOLOGICAL CONDITIONS OF THE SITE

Annual measurements of speed and direction of wind were made at the site within the context of UNDP project on wind power. Measurements were performed in accordance with international standards in the field of measuring wind speed in order to estimate wind potential (IEA/IEC) [2].

The distribution of wind speed and Weibull parameters at the height of 51 m (the axis of the gondola) for the site of Ereymentau WPS are shown in Figure 2.

According to the results of wind pressure measurements the diagram of seasonal distribution of wind speed was made. It demonstrates the changes in the wind flow speed by month in relation to average annual wind speed (Figure 3).



Figure 1. Current Ereymentau WPS.

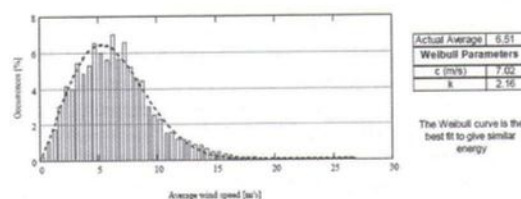


Figure 2. Distribution of wind speed and Weibull parameters at the height of 51 m.

The geological structure of this territory consists of sedimentary and metamorphic rocks of the proterozoic and the paleozoic broken by intrusions in the north-eastern part of the city, and covered by eluvial-deluvial quaternary sediments represented by clays (Figure 4).





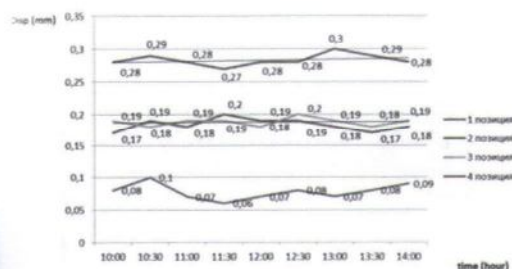


Figure 6. Graph of dependence of displacement and time.

With the help of the instrument we got the diagram of dependence of foundation absolute displacements and the time in this area (Figure 6). The diagram shows the maximum values of displacements for each position. The maximum values of displacements for the 1 position are 0,1 mm, the maximum values of displacements for the 2 and 3 positions are 0,2 mm, and 0,3 mm – for the 4 position, at the maximum wind pressure of 4,75 m/s.

The measurement results of vibration shown in the graphs of dependence of displacements, accelerations, frequencies of vibration, velocity of vibration from wind speed (Figure 7).

#### 4 DYNAMIC ANALYSIS OF WIND TURBINES ON AN ELASTIC-PLASTIC FOUNDATION

PLAXIS 2D allows modeling the interaction between soil and construction. The influence of vibration work on the soil on which it is installed was considered.

The dimensions of the soil mass are determined based on the computation of compressible stratum, represented by a clay layer of 60 m. Soil surface is set at the point  $y = 0$ . The foundation is represented by clay, which is considered here as an elastic-plastic material. A dataset is formed by selecting the *soil type* and *interface*, according to the information given in Table 1. Dynamic stiffness of soil is significantly higher than its static stiffness, as dynamic loading is usually rapid and it results in ultra-small deformations.

Material data set for footing according to the information given in Table 2.

A distributed load is applied to the foundation, as well as a horizontal force and a concentrated moment for modeling the wind turbine weight, and fluctuations that it causes. The geometric model is shown in Figure 8.

Figure 9 shows the deformed mesh of the foundation.

Figures 10, 11, 12 show the results of the accelerations, the results velocity of vibration values and the diagram of the dependence of vertical displacement on time.

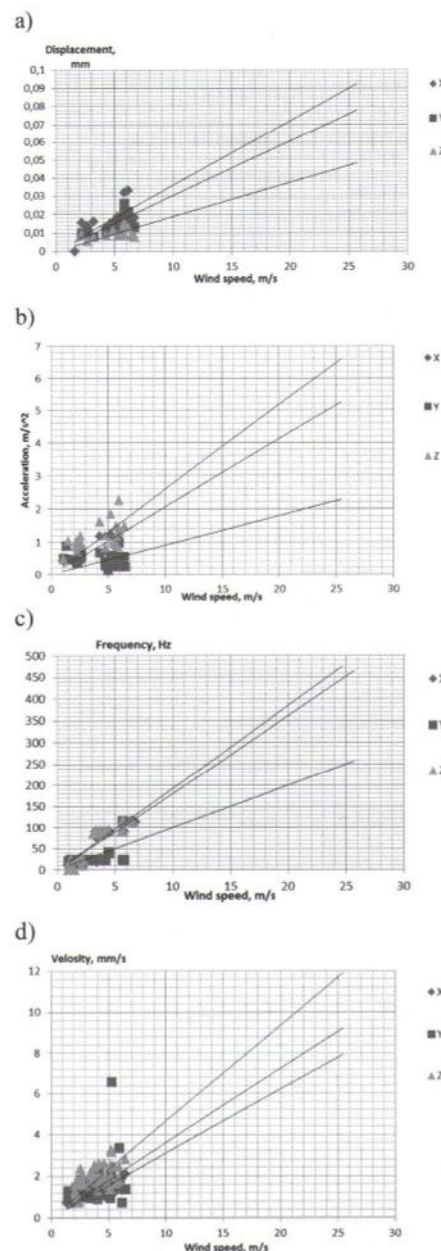


Figure 7. Graphs of dependence of displacements, accelerations, frequencies of vibration, velocity of vibration from wind speed: a) graphs of dependence of displacements from wind speed, b) graphs of dependence frequencies of vibration from wind speed, c) graphs of dependence of accelerations from wind speed, d) graphs of dependence of velocity of vibration from wind speed.

## 5 CONCLUSION

According to the measurement results the prognosis of vibration parameters of wind turbine foundation from wind pressure was made.



Table 1. Material properties of the subsoil.

Parameter	Name	Value	Unit
General			
Material model	Model	Mohr – Coulomb	–
Type of material behavior	Type	Drained	–
Soil unit weight above phreatic level	$\gamma_{\text{unsat}}$	16	kN/m <sup>3</sup>
Soil unit weight below phreatic level	$\gamma_{\text{sat}}$	18	kN/m <sup>3</sup>
Parameters			
Young's modulus (constant)	$E'$	$13 \cdot 10^4$	kN/m <sup>2</sup>
Poisson's ratio	$\nu'$	0,38	–
Initial			–
$K_0$ determination	–	Automatic	–
Lateral earth pressure coefficient	$K_{0,x}$	0,674	–

Table 2. Material properties of the footing.

Parameter	Name	Value	Unit
General			
Material model	Model	Linear elastic	–
Type of material behavior	Type	Non-porous	–
Soil unit weight above phreatic level	$\gamma_{\text{unsat}}$	24	kH/m <sup>3</sup>
Parameters			
Young's modulus (constant)	$E'$	$30 \cdot 10^6$	kH/m <sup>2</sup>
Poisson's ratio	$\nu'$	0,2	–
Initial			–
$K_0$ determination	–	Automatic	–
Lateral earth pressure coefficient	$K_{0,x}$	1	–



Figure 8. Geometric model of the foundation.

Maximum values of the parameters at the maximum wind pressure in this region are:

- 1) Maximum displacement on  $X = 0,09$  mm,  $Y = 0,076$  mm,  $Z = 0,048$  mm;
- 2) Maximum acceleration on  $X = 5,14$  m/s<sup>2</sup>,  $Y = 2,24$  m/s<sup>2</sup>,  $Z = 6,48$  m/s<sup>2</sup>;
- 3) Maximum frequencies of vibration on  $X = 450$  Hz,  $Y = 200$  Hz,  $Z = 480$  Hz;
- 4) Maximum velocity of vibration on  $X = 7,8$  mm/s,  $Y = 9$  mm/s,  $Z = 11,8$  mm/s.

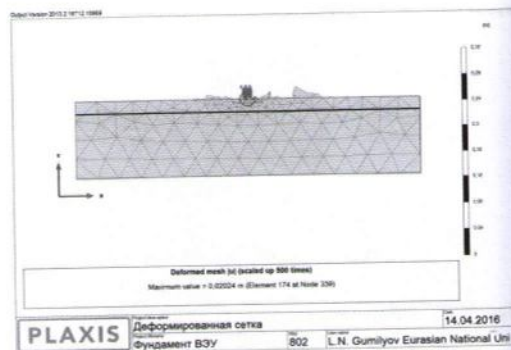


Figure 9. Deformed mesh of the foundation.

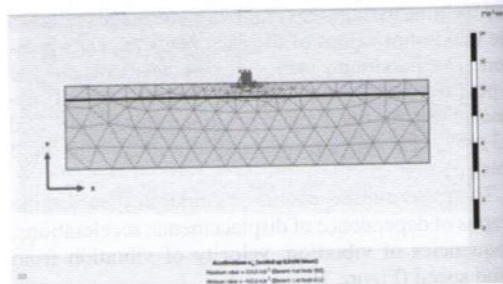


Figure 10. The results of the accelerations.

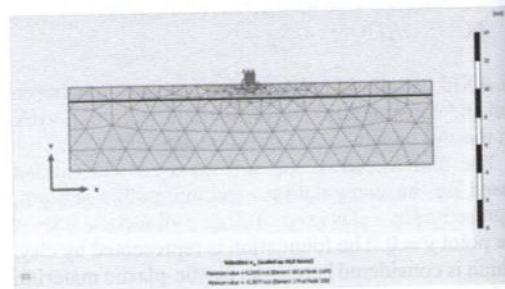


Figure 11. The results velocity of vibration values.

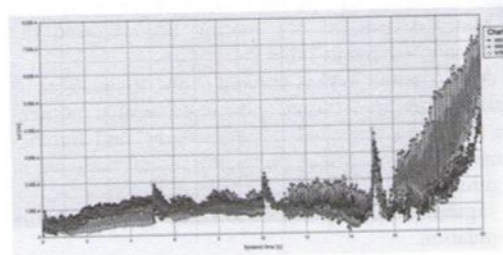


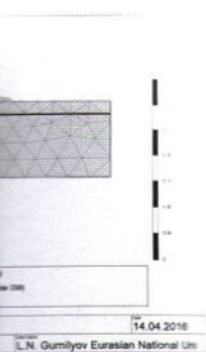
Figure 12. The diagram of the dependence of vertical displacement on time.

The obtained vibration in foundation under maximum static and dynamic loads in placement with a certain fr and vibrations speed.

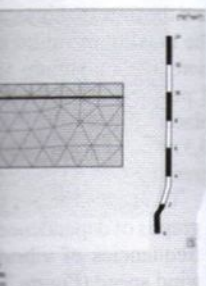
The diagram of the dependence on time shows that the exceed a limit value for W

## REFERENCES

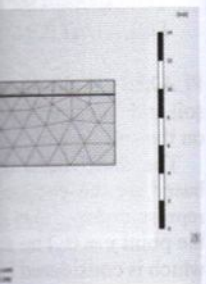
National program of wind en with the perspective till 2007, 3–5.



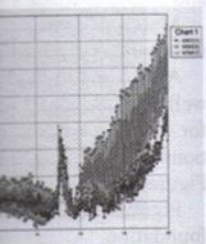
foundation.



erations.



vibration values.



dependence of vertical

obtained vibration impacts from the tower to the foundation under maximum wind pressure were integrated in Plaxis 2D software program. Having applied static and dynamic loads in the program we got the displacement with a certain frequency and acceleration, and vibrations speed.

The diagram of the dependence of vertical displacement on time shows that these displacements do not exceed a limit value for WPU foundations [3].

## REFERENCES

National program of wind energy development until 2015 with the perspective till 2024 (project) Almaty//Astana. – 2007, 3–5.

Project of the Government of Kazakhstan and the Development Program of the United Nations Organization “Kazakhstan – Initiative of the Development of Wind Power Engineering Market”, the wind power plant near Ereymentau city, Pre-investing research//Almaty. – 2008, 7–9.

DIN 45669. Human exposure to vibration in buildings, Germany, 1995–2006, p. 8.